EXHIBIT E

MID-COAST GEOTECHNICAL, INC.

Geotechnical Engineering Services

GEOTECHNICAL ENGINEERING REPORT

Proposed Bathroom Structure

Rodeo Grounds Road

Cambria vicinity of San Luis Obispo County

for:

Cambria Community Services District

Attn: Ray Dienzo

P.O. Box 65

Cambria, CA 93428

Date: November 6, 2022

Report No. 20935

File No. 22-9288

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1 INTRODUCTION

This report presents the results of our geotechnical investigation performed at the site of the proposed bathroom structure located off Rodeo Grounds Road in the Cambria vicinity of San Luis Obispo County.

The principal purpose of this investigation was to determine the geotechnical properties of the surface and subsurface soils in order to provide recommendations for general site grading and to design a suitable foundation for the proposed structure. From a geotechnical stand point the site appears to be suitable to support the proposed development when prepared as recommended herein. Research and exploratory work was conducted in accordance with presently accepted procedures consistent with the scope of work you have requested for this development. No warranty regarding the uniformity of subsurface conditions is implied.

2 SCOPE OF THE GEOTECHNICAL INVESTIGATION

The scope of our geotechnical investigation consisted of the following:

a. 2 truck mounted auger borings extended to depths of 10 to 15 feet. The location and identification of the excavations are shown on the attached drawings.
b. Observing existing man-made and natural field conditions.
c. Obtaining and testing representative bulk and undisturbed soil samples and logging the formations encountered.
d. Analysis of the field observations and laboratory testing.

3 SUMMARY OF FINDINGS

3.1 SITE DESCRIPTION

The proposed bathroom structure will be situated west of the dog park parking lot on a generally flat grass covered area. Rodeo Grounds Road borders the site on the north. Santa Rosa Creek is located about 300 feet north of the proposed site.

3.2 EXPANSIVE NATURE OF THE SOIL

The surface soils are a medium expansive silty material that are underlain with medium expansive clayey soils.

3.3 EXISTING SOIL CONDITIONS

The soft silty surface soils were encountered to 2 to 2.5 feet below grade and are underlain with firm to stiff clayey material.

3.4 GROUND WATER CONDITION

At the time of our investigation no ground water was encountered in the excavations to a depth of 15 feet below existing grade. Fluctuation in the level of the ground water may occur due to variation in rainfall, temperature, or other factors.

3.5 SEISMIC PARAMETERS

We have reviewed the available information regarding the site locations and soil type. The purpose of our review was to determine the appropriate seismic parameters for the 2019 CBC Section 1613 requirements. The site is underlain with stiff clayey material according to the logs available from the area. Density of the soil with respect to blow counts, shear strength, or shear wave velocities to a depth of 100 feet, has not been determined. However, due to the stiff nature of the near-surface material and the high blow counts in the upper 15 feet of material, we recommend that a site class D be used for the project.

Based on information provided in IBC Figures 1613.2.1 (1) and (2) and Tables 1613.2.3, the maximum considered earthquake spectral response acceleration values, adjusted for site class effects, are as follows:

short periods: S_{DS} = 0.818g

1-second periods: S_{D1} = 0.586g

(Confirmed with the USGS website www.hazards.atcouncil.org)

3.6 LIQUEFACTION POTENTIAL

We have reviewed the subsurface data to provide an opinion regarding the liquefaction potential of the site. Exploratory borings were extended 15 feet deep in the building area. In both excavations, stiff clayey material was noted within about 5 feet of the existing grade. In addition, no groundwater was observed in either test boring.

Liquefaction is generally considered a result from development of pore pressure in loose saturated sandy soil during a seismic event. The potential of liquefaction is based on the seismicity of the site, the presence and depth of groundwater, the presence of sandy soil, and density of the soil. We have not extended an exploratory boring more than 15 feet deep in the building area. Based on the presence of stiff clayey material at a depth of about 5 feet and no groundwater in the area, it is our opinion that the likelihood of liquefaction of the site is very low.

4 GRADING RECOMMENDATIONS

The following recommendations are made based on your representations that a modular bathroom structure will be constructed on the site. It is your responsibility to notify the geotechnical engineer of any changes to the proposed development. If changes occur, the recommendations contained in this report will be reviewed and may need to be revised.

4.1 GENERAL GRADING RECOMMENDATIONS

a. All grading work should be done in a professional manner and in conformance with the current local jurisdiction's grading ordinances and per the grading recommendations stated herein. In addition, all grading work shall be observed by the geotechnical engineers representatives.

b. All removal areas and foundation excavations shall be observed by the representative of the geotechnical engineer before any fill or steel reinforcement is placed. It is your responsibility to notify MID-COAST Geotechnical, Inc. when grading operations or construction begins so that the required observations can be made.

c. All surface vegetation and debris shall be removed from the work area as grading operations begin.

d. A careful search shall be made for subsurface debris and abandoned water wells, septic tanks, etc., during grading operations. If any such subsurface cavities are encountered, they shall be removed down to the firm underlying soil and properly backfilled and compacted as directed by a representative of the geotechnical engineer.

e. Special inspections should be performed in accordance with Table 1705.6 below:

TABLE 1705.6
REQUIRED VERIFICATION AND INSPECTION OF SOILS

VERIFICATION AND INSPECTION TASK	CONTINUOUS DURING TASK LISTED	PERIODICALLY DURING TASK LISTED
1. Verify materials below footings are adequate to achieve the design bearing capacity.	-----	X
2. Verify excavations are extended to proper depth and have reached proper material.	-----	X
3. Perform classification and testing of controlled filled materials.	-----	X
4. Verify use of proper materials, densities and lift thicknesses during placement and compaction of controlled fill.	X	-----
5. Prior to placement of controlled fill, observe subgrade and verify that the site has been prepared properly.	-----	X

*2019 California Building Code

4.2 BUILDING AREA REMOVAL DEPTH

The existing surface soils and any uncertified fill material underlying the proposed building area shall be removed to a minimum depth of 36 inches below the existing grade as observed at the time of our field investigation or down to firm natural material, whichever is deeper.

NOTE: Certified structural fill is required for any fill to be used for an engineering purpose. All fill beneath structures, on slopes greater than 5:1, in embankments or other

earthen structures, must be certified structural fill. All other fills, deeper than 12 inches, not to be used for structural support should be compacted but in some cases may not require certification. These uncertified fill areas, such as landscape fills, must be approved by this office and the local jurisdiction prior to grading.

4.3 GENERAL SITE REMOVAL DEPTH

In those areas outside the building area where certified fill will be placed, the surface soils shall be removed to a minimum depth of 24 inches below the existing ground surface as observed at the time of our field investigation or down to firm natural material, whichever is greater. (This removal is applicable to driveways, yards, and all other areas to be certified and receive structural fill.)

4.4 BOTTOM CHECK AND PROCESSING OF REMOVAL AREA

Field observations will be required to confirm that the removal bottom has been established in firm natural material prior to processing operations.

The exposed material at the bottom of the removal areas shall then be properly prepared and brought to near optimum moisture content before any fill is placed. The removal shall extend a minimum of 5 feet beyond the foundation perimeter or equal to the depth of removal, whichever is greater. The removed soil may be used as backfill providing all the deleterious materials, if any, are picked out.

4.5 PLACEMENT OF FILL

All fill shall be placed in layers approximately 8 inches in depth, brought to a moisture content near optimum moisture content, and compacted to a minimum of 90 percent of the maximum density (ASTM D1557) up to final pad subgrade. Fill compacted at high moisture content may be subject to yielding. Yielding or pumping grades will not be approved by this office. Material placed as certified fill shall be free of debris and rocks greater than six inches in width across the widest point.

4.6 IMPORT MATERIAL

All imported material, if any, to be used for structural fills shall be observed and approved by a representative of the geotechnical engineer prior to transport to the site. Imported fill material shall be free of debris and rocks greater than six (6) inches at the widest point. Imported soils shall be similar or less expansive than those existing on the site. The rock-to-soil ratio of the import material shall not exceed 50 percent.

4.7 SITE DRAINAGE

Small ponds of water near any structure should be eliminated. Final grading shall provide a positive drainage away from the footings. If a swale is required to collect the flow, the swale bottom should preferably be at least 5 feet from the footings or outside of the foundation wall backfill and sloped sufficiently to direct the runoff away from the building area and lot. All pad and roof drainage should be collected and transferred away from the buildings in non-erosive devices. Proper drainage shall also be provided away from the building footings and from the lot during construction. This is especially important when construction takes place during the rainy season. All drainage plans should also be in compliance with the local jurisdictions grading requirements.

5 FOUNDATION DESIGN RECOMMENDATIONS

The foundation design recommended below shall be confirmed or modified, if necessary, after grading operations are completed, depending upon the nature of the soils resulting on the surface of the graded building pad.

5.1 SOIL EXPANSION POTENTIAL

At a minimum, any foundation design should take into consideration construction on soils in the expansion index range of 51 to 90. The actual expansion index range may vary depending on the nature of the soil resulting after the completion of grading. Structural details of any foundations, such as footing thickness, concrete strength and the amount of

reinforcement should be established by your structural engineer. Due to the expansive nature of the on-site soils, we recommend the placement of at least 12 inches of compacted, non-expansive import material beneath all slabs on grade.

5.2 BEARING CAPACITY: CONTINUOUS FOOTINGS

Continuous footings supported on certified fill material are adequate for foundation support of the proposed structure and may be designed using a bearing pressure of 1500 psf. The footing depth should meet the minimum recommendations noted above. The recommended bearing values are based on an assumed embedment of a minimum of 18 inches into certified fill material and be a minimum of 12 inches wide. A 5 percent increase of bearing pressure values is allowable for each additional 6-inch increment of width or depth up to a maximum value of 2500 psf.

5.3 BEARING CAPACITY: INDEPENDENT FOOTINGS

Independent footings supported on certified fill material may be designed using a bearing pressure of 1750 psf. They shall be a minimum of 15 inches in width and extend a minimum of 18 inches below the lowest adjacent grade. A 5 percent increase is allowable for each additional 6 inches of width and/or depth up to a maximum value of 3000 psf.

5.4 FOUNDATIONS NEAR SLOPES

All foundations excavated on or adjacent to any existing or proposed slopes will require a minimum 10 feet horizontal distance to daylight. The horizontal distance is measured from the bottom of the footings to daylight on the slope or to the extent of the competent material on the slope, i.e., all slough or loose material on the slope will be discounted when measuring the distance to daylight.

5.5 WIND AND SEISMIC LOADS

The bearing pressures given are for the total of dead and frequently applied live loads and may be increased by one-third for short duration loading which includes the effects of wind or seismic forces.

5.6 PASSIVE AND FRICTIONAL RESISTANCE

Resistance to lateral loading may be provided by friction acting at the base of foundations and by passive earth pressure. An allowable coefficient of friction of 0.35 may be used with the dead load forces in the certified fill material.

Passive earth pressure may be computed as an equivalent fluid having a density of 350 pcf with a maximum earth pressure of 1750 psf. When combining passive and friction for lateral resistance, the passive component should be reduced by one-third.

5.7 ESTIMATED SETTLEMENT

Based upon test results, field observations, and compliance with these recommendations, a total settlement of less than 1 inch and differential settlement of less than 1/2 inch is expected in a distance of 20 feet.

6 ADDITIONAL RECOMMENDATIONS

6.1 PERIMETER SLABS AND GARDEN WALLS

Perimeter slabs (walkways, patios, etc.) and garden walls shall be designed as free-floating and independent of the adjacent structure. Subgrade materials in areas to receive slab-on-grade shall be prepared and presaturated as per the "Foundation" recommendations provided herein.

6.2 COMPACTION OF EXCESS SOIL

Soils generated during footing excavation operations should not be placed across the pads unless the materials are compacted to at least 90 percent of the maximum density (ASTM D1557). This also applies to sand, agricultural, and landscape fill exceeding 12 inches in depth. Compaction tests should be taken in additional fills placed in order to confirm that the minimum relative compaction requirements are achieved. It is your responsibility to notify MID-COAST Geotechnical, Inc. if testing is needed.

6.3 ROOF GUTTERS AND DOWN SPOUTS

We advise that gutters and down spouts be installed on all buildings as a means of improving the flow of run-off away from the foundation and building area. Gutters and down spouts are of particular importance when the structure is located on expansive soil, on sandy soil underlain with low permeability material, on structures with subterranean areas, or other conditions which may be sensitive to excess moisture. Down spouts should be connected to PVC pipe and drained to an approved drainage course such as a street or storm drain.

7 OBSERVATIONS AND TESTING

All foundation excavations should be approved by this firm prior to placing concrete or any steel reinforcement. All removal excavation bottoms shall be observed and approved by a geotechnical engineer or his representative prior to placement of backfill. Any fill placed for engineering purposes should be tested and certified.

Temporary wall excavations should be observed by a representative of this firm. It is your responsibility to notify MID-COAST Geotechnical, Inc. at each stage of the excavations so that observations can be made. If the examination reveals any hazard, appropriate treatment will be recommended. Please advise this office at least 24 hours prior to any required observations.

8 LIMITS AND LIABILITY

Please be aware that our contract fee for our services to prepare this report do not include additional work which may be required such as grading observation and testing, footing observations, presaturation observations, etc. Since the extent of grading and the amount of involvement of our services varies for each project, our services are normally billed on an hourly rate or per-test basis.

This report provides recommendations and comments in accordance with currently accepted practice applicable to the scope of your project. Further requirements may be imposed by the reviewing agency or necessary as a result of changes to your building or grading plans. Where additional services are requested or required, you will be billed for any equipment costs and on an hourly basis for consultation or analysis.

All documents, including maps, plans, drawings, specification and test results which we prepare or furnish or which are prepared or furnished by our independent professional associates and consultants pursuant to this agreement are considered instruments of service with respect to the project, and we will retain an ownership and property interest therein, whether or not the project is completed. Without limiting the foregoing, we reserve the right to make use of all information obtained in the performance of our services in projects for other clients, including without limitation, the right to use all test results and reports in performing services for future owners of your property.

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The limits of our liability for data contained in this report and our warranty are presented on the following page.

This report is issued with the understanding that it is the responsibility of the owner, or his representative, to assure that the information and recommendations contained herein are called to the attention of the designers and builders for the project.

Respectfully submitted,
MID-COAST Geotechnical, Inc.

Dane C. Jensen, RCE C60675
Expiration Date 12/31/2022

DCJ/maj
CCSD (1 + e-mail)
Civil Design Studio (e-mail)

9 WARRANTY

This report is based on the development plans provided to our office. In the event that any significant changes in the design or location of the structure(s) as outlined in this report are planned, the conclusions and recommendations contained in this report may not be considered valid unless the changes are reviewed and the conclusions of this report are modified or approved by the geotechnical engineer.

The subsurface conditions, excavations, and characteristics described herein have been projected from individual borings or test pits placed on the subject property. The subsurface conditions, excavation, and characteristics shown should in no way be construed to reflect any variation which may occur between these borings or test pits.

It should be noted that fluctuations in the level of the groundwater may occur due to variation in rainfall, temperature, changes in drainage and grading, and other factors not evident at the time measurements were made and reported herein. MID-COAST Geotechnical, Inc. assumes no responsibility for variations which may occur across the site.

If conditions encountered during construction appear to differ from those disclosed, this office shall be notified so as to consider the need for modifications. No responsibility for construction compliance with the design concepts, specifications or recommendations is assumed unless on-site construction review is performed during the course of construction which pertains to the specific recommendations contained herein.

This report has been prepared in accordance with generally accepted practice. No warranties, either express or implied, are made as to the professional advice provided under the terms of the agreement and included in this report.

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APPENDIX

(Laboratory results, maps, and logs)

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10 LABORATORY TEST RESULTS

10.1 MOISTURE-DENSITY DETERMINATIONS

Maximum Density-Optimum Moisture data were determined in the laboratory from soil samples using the ASTM D-1557-12e1 Test Method.

SOIL TYPE	SOIL DESCRIPTION	MAXIMUM DRY DENSITY (lbs/cu.ft)	OPTIMUM MOISTURE (%)
C1	Dark olive sandy CLAY	112.5	16.5

10.2 FIELD DENSITY SUMMARY (Ring Density Method)

TEST NO.	DEPTH (FT)	SOIL TYPE	FIELD MOISTURE CONTENT (%)	DRY DENSITY (lbs/cu.ft)	% OF MAX. DRY DENSITY
B1	2	C1	20.6	97.0	86
B1	5	C1	21.9	100.0	89
B1	10	C1	26.1		
B1	15	C1	26.2		
B2	2	C1	18.8		
B2	5	C1	21.9	99.7	89
B2	10	C1	25.0		

10.3 EXPANSION INDEX TEST

Each Expansion Index Test was performed on a representative bulk sample of the soil collected during our investigation. Expansion index test procedure is performed in accordance with ASTM D4289-19. The results follow:

SOIL TYPE	LOCATION	EXPANSION INDEX
M1	B1 @ 0-2'	55
C1	B1 @ 2-15'	88

EXHIBIT E

Midcoast Geotechnical, Inc
3124 El Camino Real
Atascadero CA 93422
Telephone: 805-461-0965

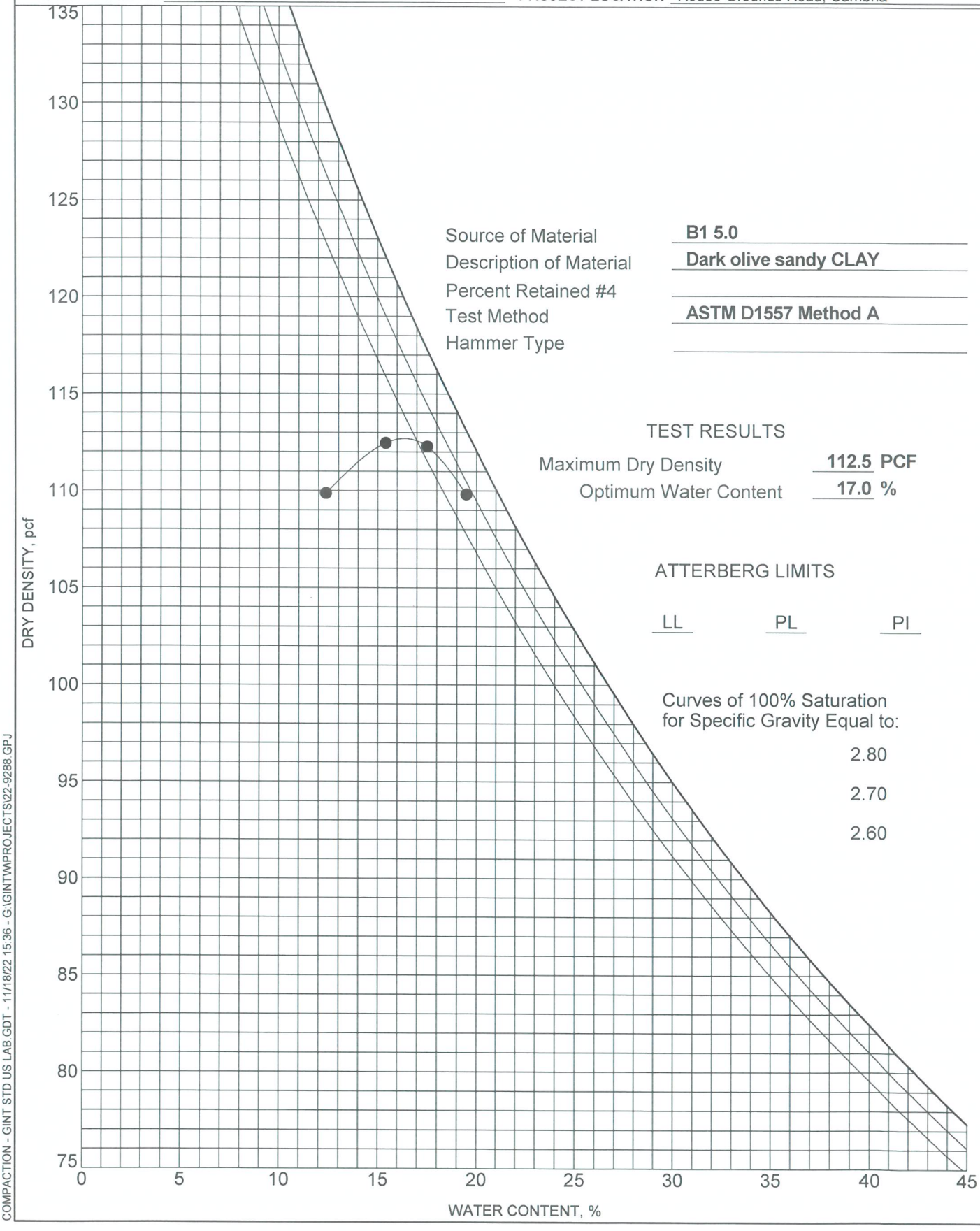
MOISTURE-DENSITY RELATIONSHIP

CLIENT Cambria Community Services District

PROJECT NAME Proposed Bathroom Structure

PROJECT NUMBER 22-9288

PROJECT LOCATION Rodeo Grounds Road, Cambria



Source of Material: B1 5.0

Description of Material: Dark olive sandy CLAY

Percent Retained #4:

Test Method: ASTM D1557 Method A

Hammer Type:

TEST RESULTS

Maximum Dry Density: 112.5 PCF

Optimum Water Content: 17.0 %

ATTERBERG LIMITS

LL PL PI

Curves of 100% Saturation for Specific Gravity Equal to:

2.80

2.70

2.60

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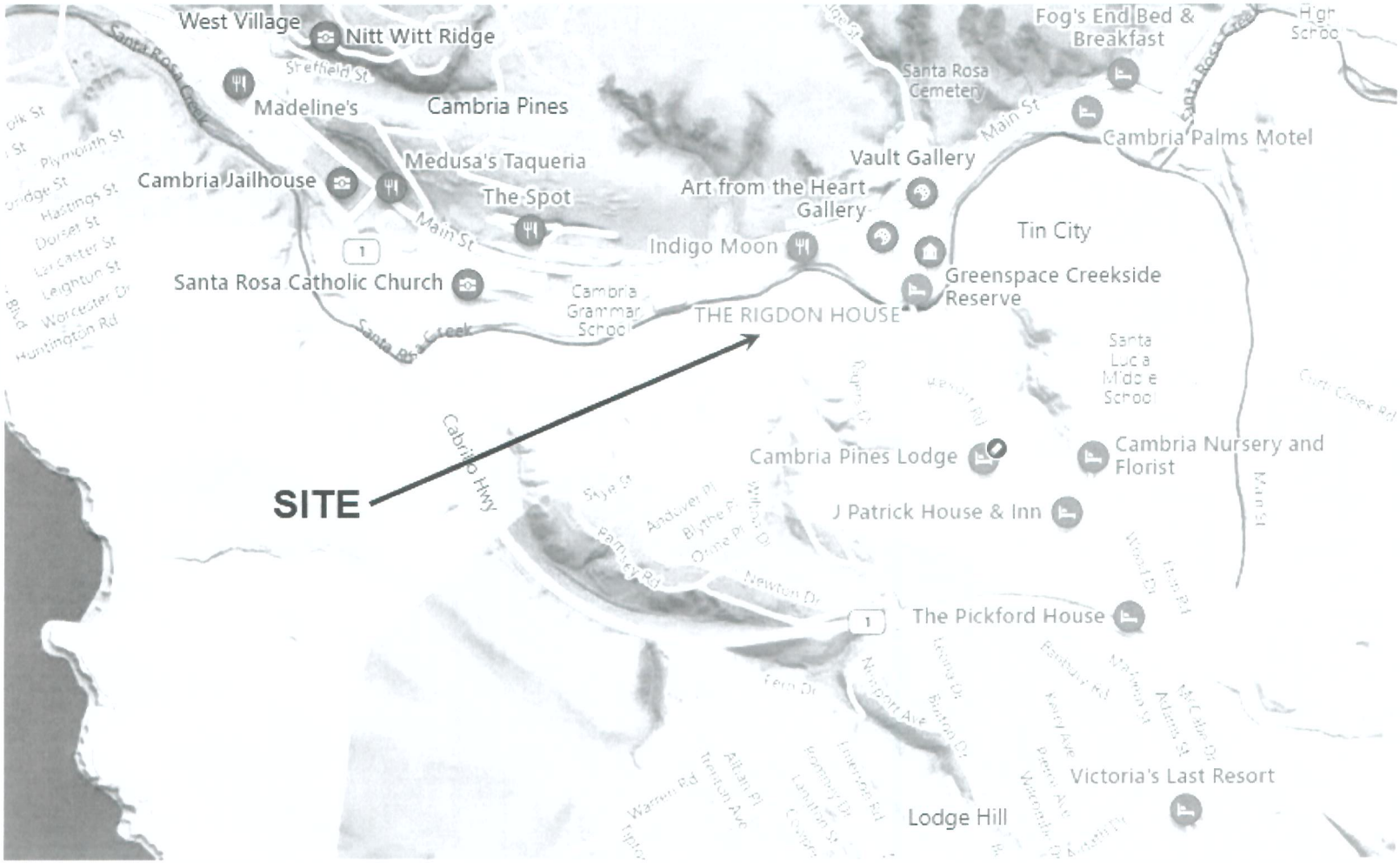
EXHIBIT E

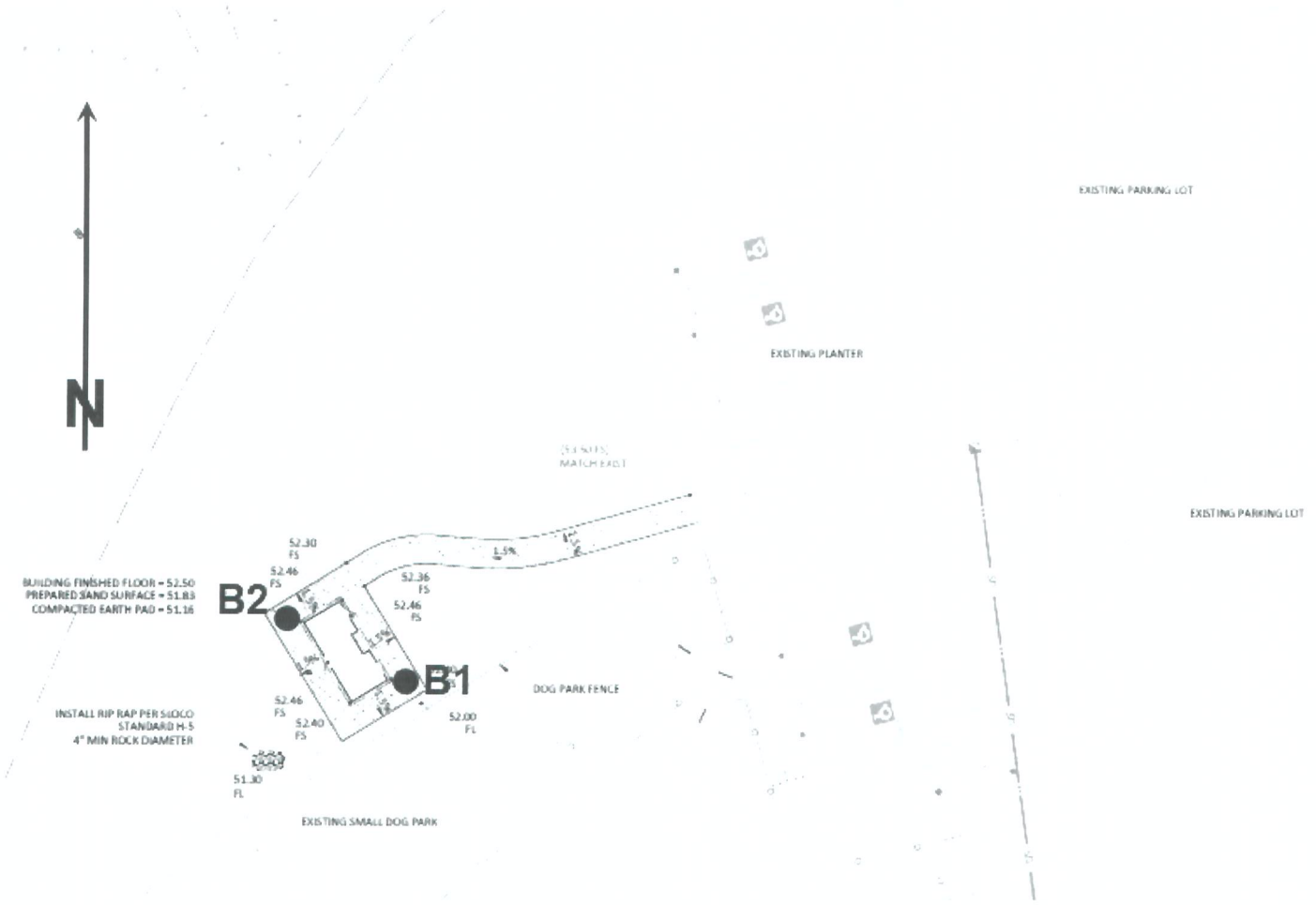


Vicinity Map **N.T.S.**

*MAP PROVIDED BY BING MAPS

MID-COAST GEOTECHNICAL, INC.	**Cambria Community Services District** **Proposed Bathroom Structure** **Rodeo Grounds Road** **Cambria vicinity of San Luis Obispo County** **California**	**Date: November 6, 2022** **Plate 1**

EXHIBIT E



Legend **N.T.S.**

- **Exploratory Soils Boring**

*PLAN PROVIDED BY OTHERS

MID-COAST GEOTECHNICAL, INC.	**Cambria Community Services District** **Proposed Bathroom Structure** **Rodeo Grounds Road** **Cambria vicinity of San Luis Obispo County** **California**	**Date: November 6, 2022** **Plate 2**

EXHIBIT E

LOG OF BORING B1

MID-COAST GEOTECHNICAL, INC.
3124 El Camino Real Atascadero CA 93422
Telephone: 805-461-0965

CLIENT: Cambria Community Services District
PROJECT: Proposed Bathroom Structure
LOCATION: Rodeo Grounds Road, Cambria
NUMBER: 22-9288
DATE(S) DRILLED: 8/2/2022 - 8/2/2022

DRILLING METHOD(S):
Truck Mounted Drilling Rig

GROUNDWATER INFORMATION:
No Ground Water Detected

SURFACE ELEVATION:

FIELD DATA			LABORATORY DATA								CLASS.			
DEPTH (FT)	SAMPLES	N: BLOWS/FT P: TONS/SQ FT T: TONS/SQ FT PERCENT RECOVERY/ ROCK QUALITY DESIGNATION	MOISTURE CONTENT (%)	DRY DENSITY POUNDS/CU.FT	RELATIVE COMPACTION (%)	ATTERBERG LIMITS LIQUID LIMIT LL	ATTERBERG LIMITS PLASTIC LIMIT PL	ATTERBERG LIMITS PLASTICITY INDEX PI	EXPANSION INDEX	MINUS NO. 200 SIEVE (%)	USCS	SOIL SYMBOL	SOIL TYPE	DESCRIPTION OF STRATUM
1									55				M1	Dark brown sandy SILT, soft, damp to moist
2		N = 38	21	97	86									Dark olive sandy CLAY, firm to stiff, moist
3														
4														
5		N = 17	22	100	89				88					
6														
7														
8													C1	
9														
10			26											
11														
12														
13														
14														
15			26											Boring Terminated at 15 feet

N - STANDARD PENETRATION TEST RESISTANCE
P - POCKET PENETROMETER RESISTANCE
T - POCKET TORVANE SHEAR STRENGTH

REMARKS:
Boring was backfilled with auger clippings

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EXHIBIT E

LOG OF BORING B2

MID-COAST GEOTECHNICAL, INC.
3124 El Camino Real Atascadero CA 93422
Telephone: 805-461-0965

CLIENT: Cambria Community Services District
PROJECT: Proposed Bathroom Structure
LOCATION: Rodeo Grounds Road, Cambria
NUMBER: 22-9288
DATE(S) DRILLED: 8/2/2022 - 8/2/2022

DRILLING METHOD(S):
Truck Mounted Drilling Rig

GROUNDWATER INFORMATION:
No Ground Water Detected

SURFACE ELEVATION:

FIELD DATA			LABORATORY DATA							CLASS.			DESCRIPTION OF STRATUM
DEPTH (FT)	SAMPLES	N: BLOWS/FT P: TONS/SQ FT T: TONS/SQ FT PERCENT RECOVERY/ ROCK QUALITY DESIGNATION	MOISTURE CONTENT (%)	DRY DENSITY POUNDS/CU.FT	RELATIVE COMPACTION (%)	ATTERBERG LIMITS LIQUID LIMIT LL	ATTERBERG LIMITS PLASTIC LIMIT PL	ATTERBERG LIMITS PLASTICITY INDEX PI	EXPANSION INDEX	MINUS NO. 200 SIEVE (%)	USCS	SOIL SYMBOL	SOIL TYPE	
1													M1	Dark brown sandy SILT, soft, damp to moist
2			19											
3													C1	Dark olive sandy CLAY, firm to stiff, moist
4														
5		N = 18	22	100	89									
6														
7														
8														
9														
10			25											Boring terminated at 10 feet

N - STANDARD PENETRATION TEST RESISTANCE
P - POCKET PENETROMETER RESISTANCE
T - POCKET TORVANE SHEAR STRENGTH

REMARKS:
Boring was backfilled with auger clippings

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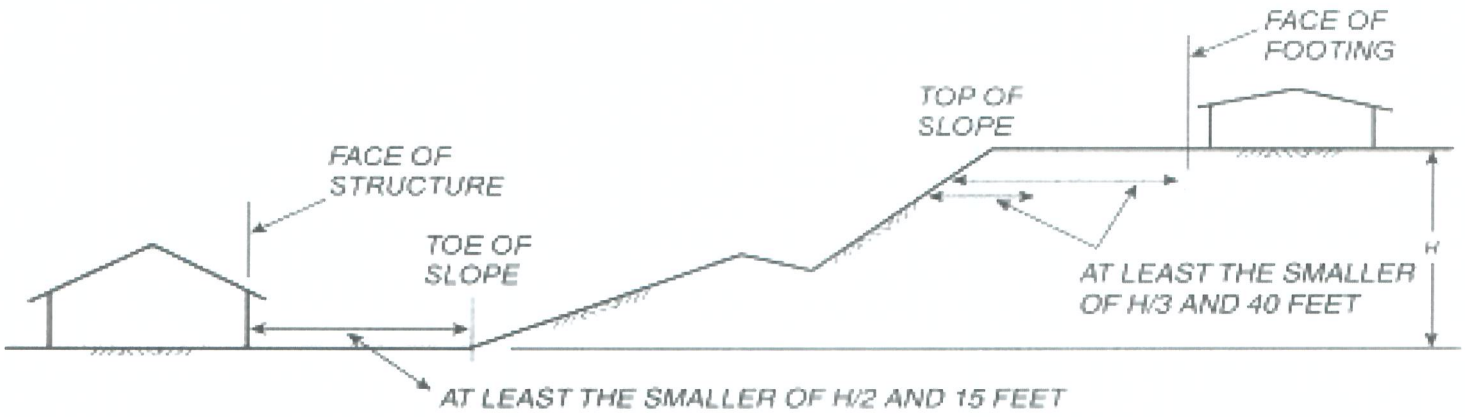
EXHIBIT E



For SI: 1 foot = 304.8 mm.

FIGURE 1808.7.1

FOUNDATION CLEARANCES FROM SLOPES

MID-COAST GEOTECHNICAL, INC.	**Slope Setbacks - Per 1808.7.1 CBC 2019**	**Plate T**